

## Fault zone strength and failure criteria

Chris Marone

Department of Earth, Atmospheric, and Planetary Sciences, Massachusetts Institute of Technology, Cambridge

**Abstract.** This paper discusses Coulomb failure criteria for brittle deformation of intact rock and fault gouge. Data are presented from laboratory experiments designed to identify the critical gouge layer thickness required to effect a transition from the standard Coulomb criterion to a modified failure law (referred to as Coulomb plasticity) appropriate for simple shear of a gouge layer. Experiments were carried out using tension fractures and quartz powder to simulate granular fault gouge. Fractures sheared without gouge obey the standard Coulomb law. A 0.6mm-thick gouge layer was required to effect the transition to Coulomb plasticity. I test and reject the hypothesis that fault zone strength and apparent coefficient of internal friction can be predicted from fracture of intact rock simply by accounting for differences in the failure laws and without considering variations in the Coulomb parameters. The data presented indicate that the stress state required for Coulomb plasticity is not developed within very thin gouge layers. This work implies that brittle fault zones have lower friction than predictions based on the strength of intact rock. However, the magnitude of this weakening effect is small (for example, a coefficient of sliding friction of 0.75 would be reduced to 0.6) and thus it is not an independent explanation of the apparent weakness of mature faults.

### Introduction

A central goal in fault mechanics has been to understand why mature faults appear significantly weaker than estimates based on Byerlee's Law. Byerlee's Law predicts coefficients of sliding friction of 0.6 to 0.8 and fault strengths of  $\geq 100\text{MPa}$  at seismogenic depths, whereas friction on mature faults is inferred to be 0.3 or less [e.g., Rice, 1992]. Immature faults (those with poorly developed gouge zones and little recent slip) do not appear to be weak.

Explanations for the weakness of mature faults include the existence of weak material within the fault zone, high fluid pressure, and dynamic effects which reduce friction and heat production [Rice, 1992; Chester *et al.*, 1993]. Notwithstanding the importance of these mechanisms, an additional factor exists that has received relatively little attention: differences in the failure laws, and strength, for intact rock and fault zones deforming under simple shear. The purpose of this paper is to evaluate these differences and to present measurements showing the transition between failure laws with increasing gouge thickness.

### Failure Criteria for Fault Zones and Intact Rock

For intact rock and granular material undergoing bulk deformation, shear strength  $\tau$  is described by the Coulomb criterion (Figure 1)

$$\tau = C + \sigma_n \tan \phi, \quad (1)$$

where  $C$  is cohesion (or cohesive strength),  $\sigma_n$  is normal stress, and  $\phi$  is the friction angle ( $\tan \phi = \mu_i$ , where  $\mu_i$  is the coefficient of internal friction). However, laboratory data, theoretical studies, and field observations indicate that a layer of fault gouge deforming in simple shear obeys a different failure law [Hobbs *et al.*, 1990]

$$\tau = C \cos \phi + \sigma_n \sin \phi. \quad (2)$$

Criterion (2) represents a stress state in which the maximum principal stress is  $45^\circ$  to the gouge layer boundary, irrespective of the external stress state [Hobbs *et al.*, 1990; Marone *et al.*, 1992]. This implies coaxiality between the macroscopic stress and strain rate vectors and thus criterion (2) has been referred to as Coulomb plasticity [Byerlee and Savage, 1992; Beeler and Tullis, 1994; Scott *et al.*, 1994].

A consequence of (2) is that brittle fault zones are expected to have lower friction than predictions based on the standard Coulomb law (1). That is, (Figure 1a) the normal stress dependence of the shear strength of a gouge layer  $\mu_a$  (the apparent coefficient of internal friction) and the apparent friction angle  $\phi_a$  are related to the true friction angle and coefficient of internal friction by [Hobbs *et al.*, 1990]

$$\mu_a = \tan \phi_a = \sin \phi = \sin(\tan^{-1} \mu_i). \quad (3)$$

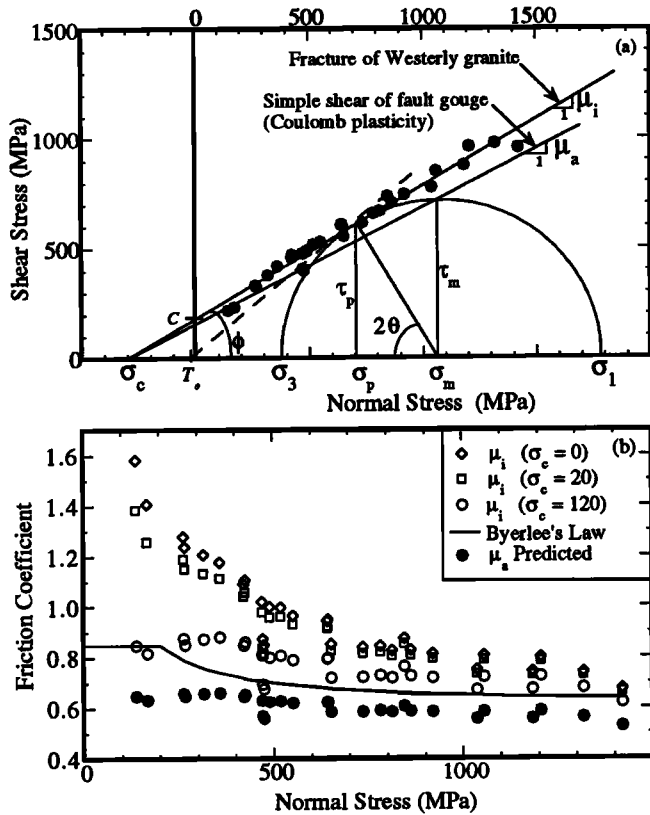
The gouge layer fails internally according to criteria (1) along Riedel shear surfaces [Mandl *et al.*, 1977] and thus the plane of the gouge layer appears to have lower friction than planes of other orientations.

Although the distinction between (1) and (2) is well defined in the extreme, the transition between them is not. The Coulomb criterion applies to intact rock and bulk deformation of granular material, whereas Coulomb plasticity applies to thick gouge zones. Does the transition require a critical gouge thickness or a critical contrast in material properties between the gouge and surrounding rock? How is the transition accomplished with fault development? In this paper I address these issues by first considering the effect of variations in  $C$  and  $\phi$  on a transition from criterion (1) to (2). I then present laboratory measurements of the frictional strength of gouge layers to examine directly the transition in failure laws.

If the Coulomb parameters are independent of the change in failure law from (1) to (2) differences in the shear strength of intact rock and fault gouge are due solely to differences in the stress state and boundary conditions *vis a vis* criteria (1) and (2). Existing data may be used to evaluate this hypothesis [Lockner and Byerlee, 1993]. Figure 1 shows fracture data for Westerly granite and the failure envelope they define. It is convenient to write the Coulomb criteria as  $\mu_i = \tan \phi = \tau_p / (\sigma_p + \sigma_c)$  where  $\tau_p$  and  $\sigma_p$  are the resolved shear and normal stresses on the fracture plane and  $\sigma_c = C / \tan \phi$ .

Copyright 1995 by the American Geophysical Union.

Paper number 95GL00268  
0094-8534/95/95GL-00268\$03.00



**Figure 1.** (a) Fracture data of *Brace et al.* [1966] and *Byerlee* [1966] for Westerly granite along with the Coulomb failure envelope they define (upper line of slope  $\mu_i$ ). Data are shear and normal stress resolved on the fracture plane at failure. The line of slope  $\mu_a$  represents the Coulomb plasticity failure law. Dashed line indicates failure criteria used by *Lockner and Byerlee* [1993] who mistakenly took  $C$  equal to  $T_o$ . (b) Coefficient of internal friction derived from the fracture data for three values of  $\sigma_c$ . The data require  $\sigma_c = 120$ MPa and thus the predicted  $\mu_a$  values are significantly below laboratory measurements of gouge strength which follow *Byerlee's* law. (Modified from *Lockner and Byerlee*, [1993])

Under these assumptions, *Lockner and Byerlee* [1993] suggested that the fracture strength for intact rock could be used to predict the strength and  $\mu_a$  for fault gouge, via equation (3). They reported agreement between  $\mu_a$  so derived and laboratory measurements of gouge strength (Figure 1). However, *Lockner and Byerlee* mistakenly identified  $\sigma_c$  as the tensile strength  $T_o$  and equated this with cohesion (their equation 1). From Figure 1,  $\sigma_c = C$  if  $\phi = 45^\circ$ , a rare case for brittle rocks. However, there is no theoretical basis for equating  $C$  with tensile strength, which is generally a factor of 5 to 10 less than  $C$ .

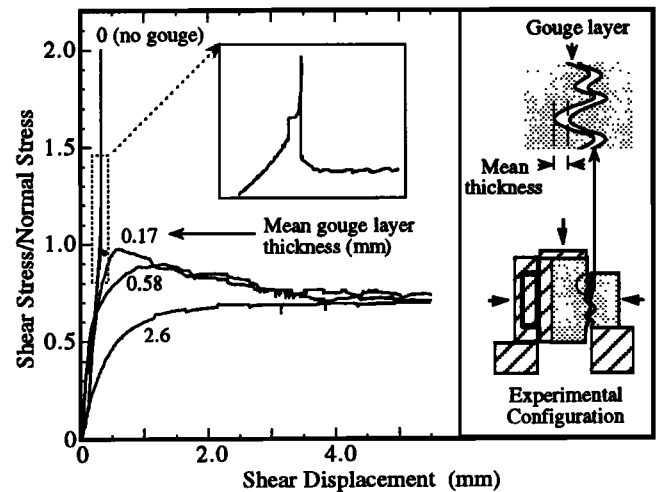
In Figure 1b, I show  $\mu_i$  for the fracture data of Figure 1a using three values of  $\sigma_c$ . *Lockner and Byerlee* used values of  $\sigma_c = 0$  [1992] and 20MPa [1993], however, these are incorrect. In his original study, *Byerlee* [1966] reported cohesion values of 70 to 170MPa, which would imply  $\sigma_c$  of 100 to 200MPa. A least squares fit of equation (1) to all the data of Figure 1a gives  $C = 191$ MPa and  $\mu_i = 0.60$ . Neglecting curvature in the failure envelope, the data require  $C$  of at least 100MPa and  $\mu_i = 0.83$ , which gives  $\sigma_c = 120$ MPa. From equation (3) this yields  $\mu_a$  values significantly below measurements of gouge strength and *Byerlee's* law (Figure 1b).

The observation that gouge strength is not predicted from the strength of intact rock indicates that  $\sigma_c$  is not the same for these materials (Figure 1). This is perhaps not surprising since cohesion differs greatly for intact and fractured rock. The derivation of equation (3) requires that  $\sigma_c$  be the same for both criteria [*Hobbs et al.*, 1990]. Hence, a better test of equation (3) and the relationship between criteria (1) and (2) would be to compare cases for which this were true. Shear of a pre-existing, natural fracture at moderate normal loads provides such a case. Failure is expected to obey (1) when the fracture is sheared from the mated (fully interlocking) position without gouge, whereas shear of a sufficiently thick gouge layer obeys (2). Cohesion for a pre-existing fracture at low normal loads can be taken as zero, hence  $\sigma_c$  is zero in both cases. Friction measurements made on increasingly thick gouge layers should show a transition from criteria (1) to (2).

### Shear of Natural Fractures

Tension fractures were produced within samples of Westerly granite and sheared at 15MPa normal stress within the direct shear apparatus of J. Dieterich at the U.S.G.S., Menlo Park, Ca. Normal stress was held constant and a roller-bearing assembly was used to isolate shear on a single fracture surface in the double direct shear geometry (Figure 2). Fractures had nominal area of 5cm x 5cm and were made approximately flat at longer wavelengths by notching the perimeter of the block to guide tensile fracture. Maximum peak-to-trough roughnesses of the surfaces were 6-8mm [*Marone and Durham*, 1992].

Fractures were sheared from the mated position with ultra-fine quartz powder (median diameter  $< 10\mu\text{m}$ ) used to simulate fault gouge. Twelve experiments were run with varying thicknesses of gouge. Stress-strain characteristics varied systematically with gouge thickness (Figure 2). For zero gouge, the stress-strain



**Figure 2.** Main plot shows friction data from four experiments in which gouge layers of different thickness were sheared between tension fractures at 15MPa of normal stress. Right inset illustrates direct-shear experimental configuration with roller-bearing-way assembly, rock (stippled), and definition of mean gouge thickness. Peak-to-trough fracture roughness was 6-8mm. Upper inset is enlargement of data for shear without gouge, showing stable post-failure sliding with sliding friction=0.95. Shear displacement has been corrected for apparatus stiffness. Second order variations in friction are related to slip velocity changes used to interrogate detailed frictional characteristics [*Marone and Durham*, 1992].

behavior resembled that for fracture of an intact sample: a steep loading curve was terminated by a sudden, audible stress drop at failure. The post-failure coefficient of sliding friction was 0.95. Thicker gouge layers showed a more gradual onset of sliding and lower peak frictional strength (Figure 2).

I take the values of peak strength as the point of macroscopic failure. Mated bare surfaces failed at a shear stress of 32MPa. Assuming standard Coulomb failure this corresponds to a coefficient of internal friction of 2.1, which from equation (3) predicts an apparent coefficient of internal friction of 0.9 for failure via Coulomb plasticity (Figure 3).

For the friction measurements involving gouge, I make no *a priori* assumption about the governing failure law. Values of  $\mu_i$  and  $\mu_a$  are calculated for both criteria (Figure 3). Measured friction for the 0.17mm-thick layer is close to  $\mu_a$  predicted from the experiment without gouge, however, the reverse is not true.  $\mu_i$  predicted from the measured value is significantly above the 2.1 value appropriate for Coulomb failure. For a slightly thicker gouge layer (0.58mm thickness), the measured friction matches that predicted for Coulomb plasticity and this value predicts  $\mu_i$  of about 2.1, in agreement with the measured value for criterion (1).

Although the preceding analysis has focused primarily on the coefficient of internal friction, the weakening effect associated with failure via Coulomb plasticity also applies to sliding friction. An estimate of this effect can be made using the steady-state friction value for shear without gouge (see inset to Figure 2), which is roughly 0.95. Applying equation (3) indicates a coefficient of friction of 0.69 for a gouge layer of sufficient thickness to deform by Coulomb plasticity. This is about equal to the sliding friction values for the 0.58 and 2.6mm-thick layers (Figure 2) and thus in agreement with the results for internal friction.

## Discussion and Conclusions

The data indicate that a gouge layer approximately 0.6mm thick is required to effect the transition from failure via the Coulomb criteria (1) to failure by Coulomb plasticity (Figure 3). A thinner layer shows transitional behavior. These observations

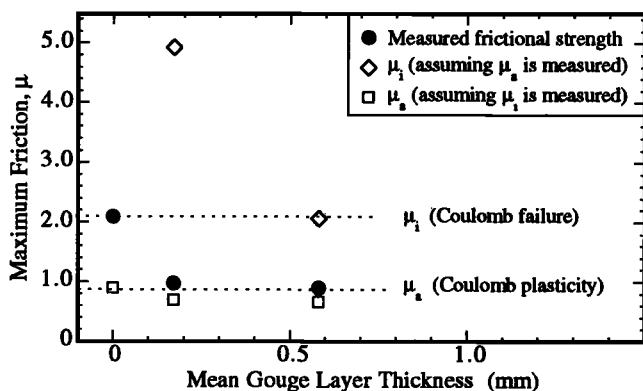


Figure 3. Maximum (peak) friction values for the data of Figure 2. The experiment with a 2.6mm-thick layer is not included because it did not show a peak stress. For shear without gouge, failure obeys criterion (1) and thus the measured friction value is the coefficient of internal friction  $\mu_i$ . From equation (3), this value predicts that failure via Coulomb plasticity will show an apparent coefficient of internal friction  $\mu_a$  of 0.9.  $\mu_i$  and  $\mu_a$  are calculated assuming that the measured friction value is  $\mu_a$  and  $\mu_i$ , respectively. The data show that Coulomb plasticity is met for a mean gouge thickness of 0.58mm.

rely on two assumptions: 1) that cohesion is approximately zero and 2) that the coefficient of internal friction is independent of gouge thickness over the range considered. The first is justified for pre-existing fractures sheared at low normal loads. The second is justified because the surface roughness is large in comparison to the layer thickness. Peak frictional strength decreased significantly when mean layer thickness, 2.6mm, was comparable to surface roughness (Figure 2).

The results indicate that the stress state required for Coulomb plasticity is not developed within exceedingly thin gouge layers, presumably due to boundary friction with the contacting rough surface. Boundary friction is expected to scale with surface roughness, and thus the transition from criteria (1) to (2) may occur at a critical ratio of layer thickness to asperity height. This ratio is about 0.1 in the present experiments, which implies a critical layer thickness of about 10 $\mu$ m in experiments using ground surfaces, which typically have maximum roughness of 100 $\mu$ m or less. Thus in all previous experiments on gouge layers known to the author failure is governed by criterion (2).

Although equations (2) and (3) follow directly from the Coulomb criteria written in terms of the mean stress and maximum shear stress, only recently has the evidence and implications of these criteria for fault zones and friction been recognized [Hobbs *et al.*, 1990; Marone *et al.*, 1992; Beeler and Tullis, 1994]. A simple geometric relationship exists between the failure criteria for intact rock and fault gouge, however, fault zone strength cannot be derived from the strength of intact rock without accounting for differences in the Coulomb parameters.

The weakening effect implied by a transition from standard Coulomb failure to Coulomb plasticity within a fault zone is not of sufficient magnitude to explain the apparent weakness of mature fault zones. However, this effect could augment other weakening mechanisms such as increased pore pressure.

**Acknowledgments.** I would like to thank J. Dieterich and B. Kilgore for use of their laboratory and D. Scott, F. Chester and three anonymous reviewers for thoughtful reviews. This work was supported by USGS award 1434-94-G-2417 and NSF grant EAR-9313082

## References

- Beeler, N. M. and T. E. Tullis, Implications of Coulomb plasticity for the velocity dependence of experimental faults, *Pure Appl. Geophys.*, in press, 1994.
- Brace, W. F., B. W. Paulding, and C. H. Scholz, Dilatancy in the fracture of crystalline rocks, *J. Geophys. Res.*, 71, 3939-3953, 1966.
- Byerlee, J. D., The frictional characteristics of Westerly granite, Ph.D. thesis, MIT, Cambridge, MA, 1966.
- Byerlee, J. D., and J. C. Savage, Coulomb plasticity within the fault zone, *Geophys. Res. Lett.*, 19, 2341-2344, 1992.
- Chester, F. M., J. P. Evans, and R. L. Biegel, Internal structure and weakening mechanisms of the San Andreas fault, *J. Geophys. Res.*, 98, 771-786, 1993.
- Hobbs, B. E., A. Ord, and C. Marone, Dynamic behaviour of rock joints, *Proc. Int. Symp. on Rock Joints*, eds. N. R. Barton and O. Stephansson, Loen, Norway, pp. 435-445, 1990.
- Lockner, D. A., and J. D. Byerlee, Limits of fault strength due to the constraints imposed by fault geometry (abstract), *Eos Trans. AGU*, 73, 511, 1992.
- Lockner, D. A., and J. D. Byerlee, How geometrical constraints contribute to the weakness of mature faults, *Nature*, 363, 250-252, 1993.
- Mandl, G., L. N. J. de Jong, and A. Maltha, Shear zones in granular material, *Rock Mech.*, 9, 95-144, 1977.
- Marone, C., and W. Durham, Double-direct-shear friction experiments using tension fractures: scaling of constitutive parameters for rock

- friction to fractally-rough surfaces (abstract), *Eos, Trans. AGU*, 73, 565, 1992.
- Marone, C., B. E. Hobbs, and A. Ord, Coulomb constitutive laws for friction: contrasts in frictional behavior for distributed and localized shear, *Pure and Appl. Geophys.*, 139, 195-214, 1992.
- Rice, J. R., Fault stress states, pore pressure distributions, and the weakness of the San Andreas fault, in *Fault Mechanics and Transport Properties of Rocks*, edited by B. Evans and T.-f. Wong, pp. 475-503, Academic Press, 1992.
- Scott, D. R., C. Marone, and C. G. Sammis, The apparent friction of granular fault gouge in sheared layers, *J. Geophys. Res.*, 99, 7231-7246, 1994.
- 
- C. Marone, Department of Earth, Atmospheric, and Planetary Sciences, Massachusetts Institute of Technology, Cambridge, MA 02139 (email: cjm@westerly.mit.edu)
- (Received August 31, 1994; revised December 21, 1994; accepted December 28, 1994.)